



### Functional Unit Choice for Comparative Pavement LCA Involving Use-Stage with Pavement Roughness Uncertainty Quantification (UQ)

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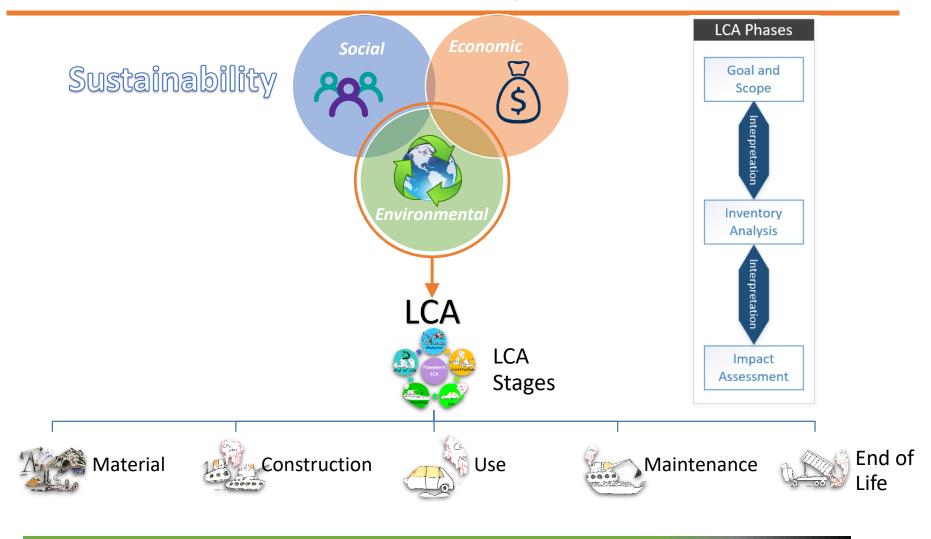
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**Other Stages** 

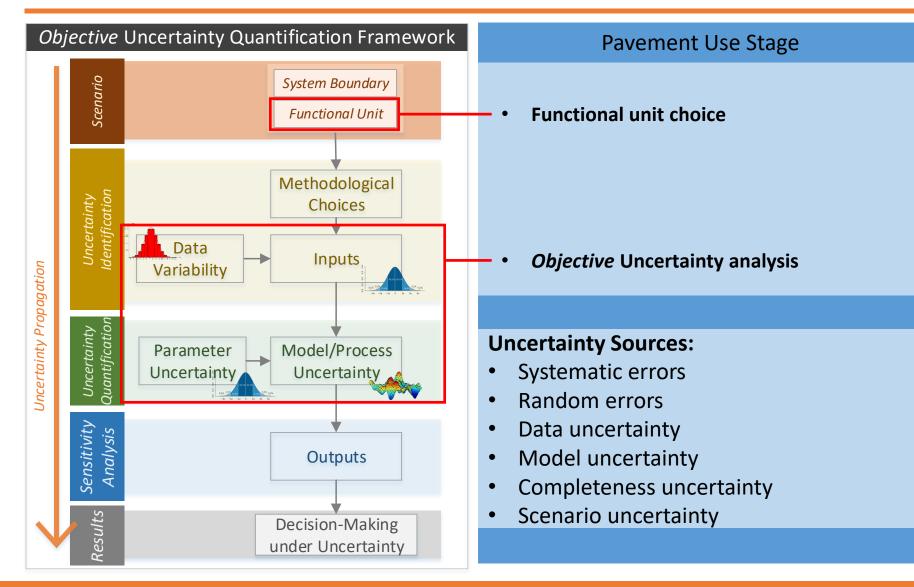
# **Sustainability & LCA**



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### Outline







# **Functional Unit (FU) Choice**

- A unit of measurement of system components to which inputs and outputs of LCA are normalized
- FHWA reference doc: "particular length of pavement with a specified geometry that meets the acceptance criteria over a specified length of time."
- Common FUs used in the literature:
  - Physical
    - Lane-mile (LM)
  - Annualized
    - Lane-mile year (LMY)
  - Structural or performance-based
    - Performance-lane-length (PCR-lane-mile)



### **FU Choice in Comparative LCA**

• FU for different design alternatives for a specific project/corridor/location.

Example FU:

• 20 lane-mile of pavement section serving 40,000 traffic for 50 years.



# **FU Choice in Comparative LCA**

- What to do for comparative LCA of <u>different</u> projects?
  - it is not straightforward for projects with different characteristic or design inputs.
  - This is the case of benchmarking studies where projects from past, for example, are compared to current designs
  - Example of Illinois Tollway pavement sustainability projects: Current projects to be evaluated against baseline projects from 1990s.
    - Not similar designs!



# **FU Choice in Comparative LCA**

- Take previous FU as example:
  - 20 lane-mile of pavement section <u>serving</u> 40,000 traffic over 50 years.
  - This fails to describe differences among projects.
- Therefore, a new FU in terms of Vehicle-lengthtraveled was defined e.g. vehicle-miles-traveled
- If the use stage accounts for the performance, then no need to include it directly in the FU.
  - Assuming poor performing pavement will result in higher impacts





### **Example Application – widening projects**

Toll Road	Year	Project Code	MP	Length (mi)	Analysis Period (yr)	AADT; % Truck	Description
Jane Addams Memorial I- 90/I-39/ US 51	2012- 2013	4077 A	49.7 to 53.6	3.9	62 yrs; 3 overlays	28,460 EB; 13.3%	Roadway widening (3 lanes 12-inch JPCP) and reconstruction
	2014	4133 B	24.9 to 33.5	8.6	62 yrs; 3 overlays	19,240 WB; 20.3%	Roadway widening (3 lanes 11.25-inch JPCP) and reconstruction
Tri-State I-94/I-294/ I- 80	2007- 2008	5228 C	15.84 to 13.24	2.6	62 yrs; 3 overlays	148,200 14.6%	Roadway widening and reconstruction (with 12-inch JPCP) from 3 to 4 lanes
Ronald Reagan Memorial I-88	1999	723 D	133.7 to 138.8	5.1	44 yrs; 2 overlays		Roadway widening and reconstruction to 3 (12-inch JPCP) lanes





# Methodology

#### Material, Construction, Maintenance, and Endof-Life Stages

A life-cycle inventory database was developed combining operational or process activity data collected with processes available in commercial software and databases such as SimaPro and US-Ecoinvent.

(Yang et al., 2016; Kang et al., 2014)









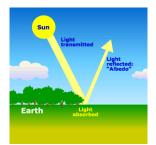




# Methodology

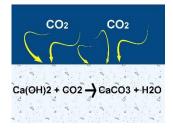
#### Use Stage

1. Albedo (Harvey, et al., 2016)



$$m_{CO2} = \sum_{n=1}^{N} 100 * (\alpha_{new}^{n} - \alpha_{ref}) * (f_{RF}) * A$$

#### 2. Carbonation (Lagerblad, 2005)



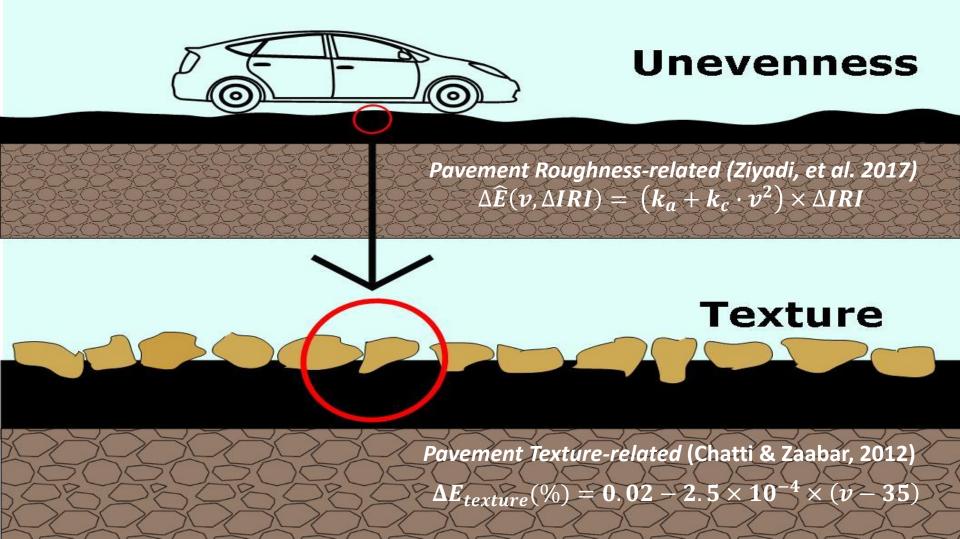
$$CO_2(kg) = k \times t^{0.5} \times c \times CaO \times r \times A \times M$$





# Methodology

3. Rolling Resistance (RR)





### **Comparison of Use Stage for Different FU**

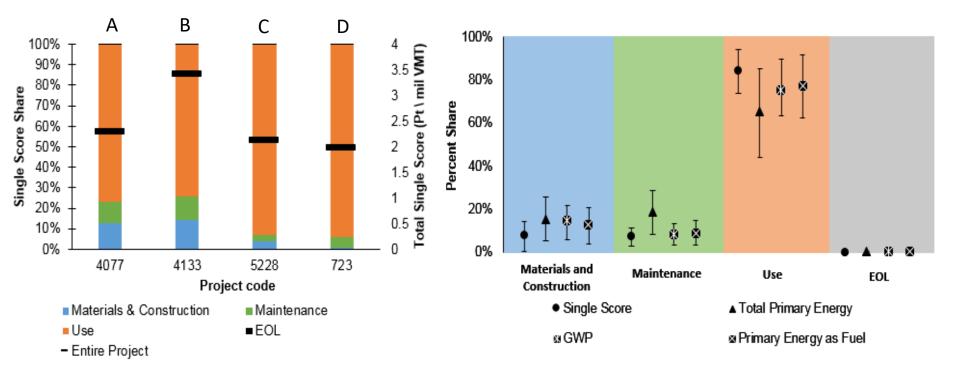
	GWP (tonne-CO <sub>2</sub> -eq)			
Output	Project Code			
	4077 A	4133 B	5228 C	723 D
Total (tonne-CO <sub>2</sub> -eq.)	38,915	70,331	129,939	132,931
Total VMT (millions)	4,199	6,354	12,451	20,921
Lane-Mile (LM) (tonne-CO <sub>2</sub> -eq. / lane-mile)	3,326	2,726	12,494	8,688
Lane-Mile Year (LMY) (tonne-CO <sub>2</sub> -eq. / lane-mile-yr)	54	44	202	197
VMT (tonne-CO <sub>2</sub> -eq. / million VMT)	9.3	11.1	10.4	6.4





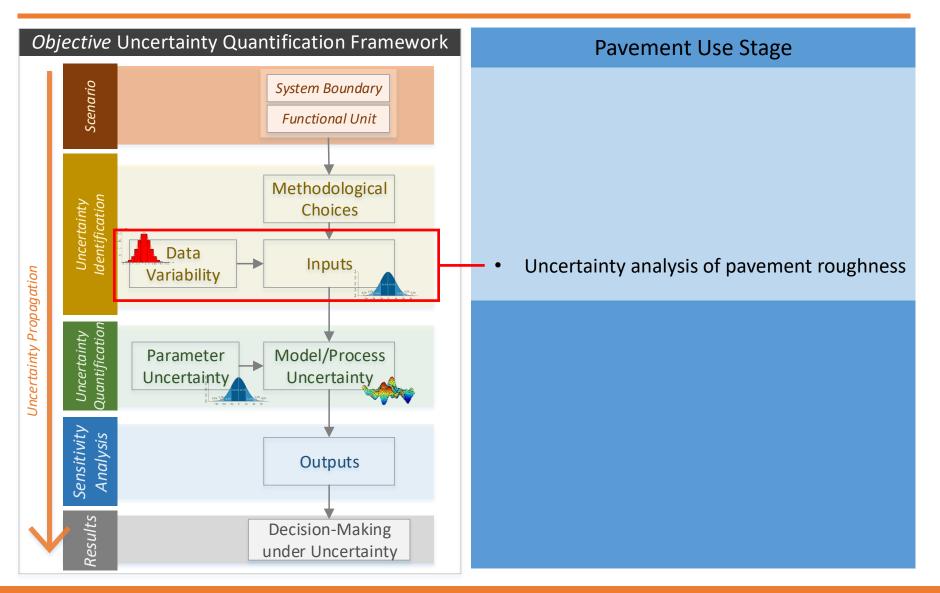
# **Range of Outputs**

 For VMT as FU the following range of results were obtained



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### **Uncertainty of Pavement Roughness**



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### **Uncertainty of Pavement Roughness**

- IRI Progression and drop model developed
- Stochastic approach for IRI uncertainty by adding random noise

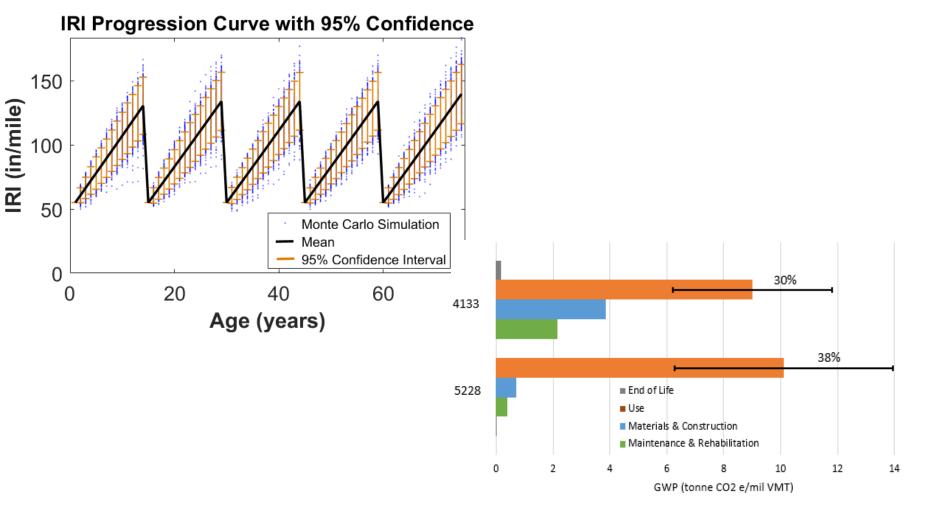
**IRI progression model:**  $IRI_t = IRI_{t-1} + a * Thickness^b * ESALs^c + \varepsilon \sim N(0, \sigma^2)$ 

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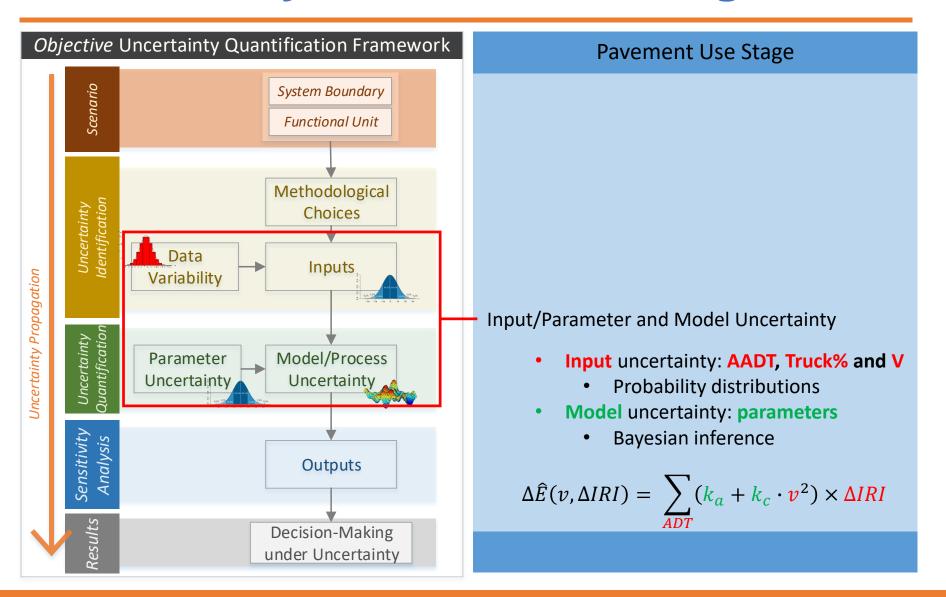
### **Uncertainty of Pavement Roughness**

#### • IRI Progression with 95% confidence interval



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# Uncertainty of Pavement Roughness







# **Projects**

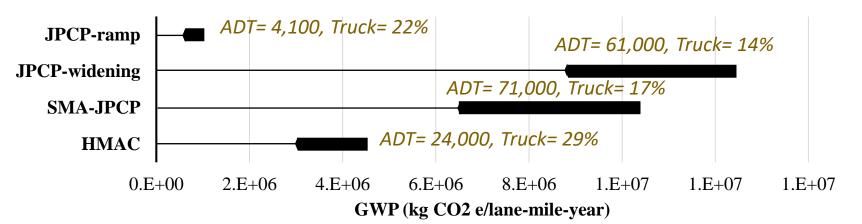
Toll Road	Milepos t	Year	Code		Analysis Period (yr)	AADT; % Truck	Description
Jane Addams Memorial I-90/I-39/ US 51	8.9 to 3.9	2008	I-08-5542 HMAC	5	75 yrs; 4 overlays	24,120 EB; 29.4% 1.47% Growth	Roadway (with 12 and 15- inch HMAC) of 3 lanes and bridge reconstruction. from Plaza 1 (SO Beloit) to IL RT 173 EB.
Tri-State I-94/I- 294/ I-80	17.3 to 30.1	2001 - 2002	RR-99-8101 SMA-JPCP	12.8	58 yrs; 4 overlays	70,864 NB, 17.1%, 0.92% G	Roadway partial resurfacing and rehabilitation (3-inch SMA overlay over 4 lanes JPCP)
Tri-State I-94/I- 294/ I-80	15.84 to 13.24	-	I-07-5228 JPCP- widening	2.6	62 yrs; 3 overlays	61,270 SB 13.8% 0.92% G	Roadway widening and reconstruction (with 12-inch JPCP) from 3 to 4 lanes
Tri-State I- 94/I-294/ I-84	7.5 to 7.8	2013 - 2014	I-12-4066 JPCP-ramp	1.2	62 yrs; 3 overlays	4,100 NB; 22.1% 0.92% G	A new highway interchange ramp (2 lanes 12-inch JPCP) construction at the junction of I-57 and I-294.

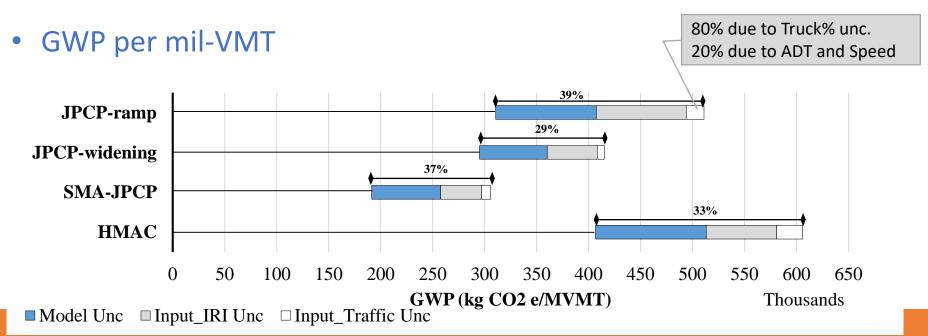




### **Use Stage - Roughness Uncertainty**

• GWP per lane-mile year









### Summary

- Functional unit choice as an important methodological choice
- Introduced Vehicle-length-traveled (e.g. VMT) as a <u>feasible</u> FU for benchmarking projects
- Importance of uncertainty quantification in LCA
- 30-40% variance in use stage results due to input/model uncertainties
- Expand the uncertainty analysis to other stages

### Thank You!

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## **Study Assumptions**

Parameter	Uncertain Quantity and Assumptions
Speed	<ul> <li>Speed distributions obtained from 2015 Traffic data report (CDM SMITH, 2015).</li> <li>Ramp section speed was assumed posted speed of 35 mph.</li> <li>Yearly average speed variation was not considered</li> </ul>
Traffic Data	<ul> <li>Average daily traffic (ADT), and percent truck traffic collected from 2015 Traffic data report (CDM SMITH, 2015) for each section. % measured error in ADT reported as (ADT ~ N(μ,0.1μ/2)).</li> <li>10% error in truck percentage. Truck composition according to WIM data from I-294 and I-94 sections (Years 2012 and 2014, Jan, Feb and Apr months): 90% large, 9% medium and 1% small truck.</li> <li>Truck classification and conversion between different systems can be found elsewhere (TRB, 2016?)</li> </ul>
Truck Loading	
Truck Loading	<ul> <li>Sample WIM data from I-294 and I-94 sections.</li> <li>Years 2012 and 2014</li> <li>Gaussian mixture model was generated from real WIM data for simulation.</li> </ul>
Temperature	- 2010 Average monthly temperature data from Chicago O'hare weather station (https://www.ncdc.noaa.gov, accessed 2/2017)
IRI	<ul> <li>- Initial IRI values of 55 in/mile for asphalt and 60 in/mile for concrete sections. Averaged from historic Tollway data.</li> <li>- Section specific IRI progression curves adopted from literature (TRB, 2016)</li> <li>- Data variances were calculated from sections with similar pavement surface type throughout the network.</li> </ul>
Material	<ul> <li>- 10% error in section surface area (Area ~ N(μ,0.1μ/2))</li> <li>- Albedo of AC: [0.15 – 0.05], PCC = [0.4 – 0.2] (Yu et al., 2013, Kaloush et al., 2008)</li> <li>- RF = [-2.9 to -1.3] (Xu, et al., 2016)</li> </ul>

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Scenario Description for	Performance (in	Traffic	Functional Unit Choice and		
Comparison	terms of IRI)		Consequences on Use-stage		
			Impact		
Alternative pavement trials	▲ IRI	<sup>Traffic</sup> A=B	LM: A > B		
for the same traffic with		A-D	LMY: A > B		
same design lives.	В		VMT: A > B		
Different expected perfor-			Consistent results when analysis		
mance (poor performance	t 1		period is the same.		
for A) with same analysis	$AP_A = AP_B$	+			
period. Different designs with dif-			LM: B > A		
ferent design lives account-		▲Traffic <sub>A</sub> B	LMI: $B > A$ LMY: most likely $B > A$		
ing for traffic volume dif-	IRI B		VMT: most likely $A = B$		
ferences.	A		<i>LM and LMY will penalize</i>		
Similar expected perfor-			longer living pavement. Results		
mance within the analysis		A	depend on change in AP and dif-		
period (shorter life for	AP <sub>A</sub> AP <sub>B</sub>		ferences in performance and		
pavement A).			traffic.		
Arbitrary selection from			LM: A > B		
network with different per-	▲ IRI	ATraffic /	LMY: $A > B$		
formance and traffic.		A /	VMT: most likely $A > B$		
Same or different design	В		- VMT will favor better perform-		
lives.		В	ing pavement B if traffic volume		
Assume A is the poor per-	t t	t	is not too low If traffic volume		
forming with higher traffic	$AP_A = AP_B$	+	is too low, indication of overde-		
and same analysis period.			sign.		
Arbitrary selection from			Total: can vary.		
network with different per-	▲ IRI	<b>▲</b> Traffic /	LM: can vary.		
formance and traffic.		в	LMY: can vary.		
Same or different design	B		VMT: most likely $A > B$		
lives.			- VMT will favor better perform-		
Assume A is the poor per-	11/1//////////////////////////////////	A	ing pavement B.		
forming with different traf-	AP <sub>A</sub> AP <sub>B</sub>	A.P(A) :: A.P(B)	- Under higher traffic conditions		
fic.		xore filosii - xore filosii	LM and LMY may penalize		
			pavement B.		

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AP = analysis period

LCA		GWP (tonne-CO <sub>2</sub> -eq)			
LCA	Component	Project Code			
Stage		4077	4133	5228	723
	Roughness-Related Passenger Vehicle	25,144	35,950	93,047	114,648
	Roughness-Related Small Truck	624	1,483	2,577	2,018
	Roughness-Related Medium Truck	824	1,957	3,400	2,662
e	Roughness-Related Large Truck	2,988	7,095	12,325	9,651
Use-stage	Texture-Related Medium Truck	1,413	3,263	5,604	5,457
Jse-	Texture-Related Large Truck	2,738	6,324	10,860	10,576
	Albedo Mainline		6,943	1,111	-8,321
	Albedo Shoulders	2,087	7,423	1,056	-3,645
	Carbonation Mainline	-55	-111	-46	-116
	Carbonation Shoulders		0	0	0
	Total (tonne-CO <sub>2</sub> -eq.)	38,915	70,331	129,939	132,931
	Total VMT (millions)	4,199	6,354	12,451	20,921
	Per Functional Unit of Lane-Mile				
	(LM)	3,326	2,726	12,494	8,688
	tonne-CO2-eq. / lane-mile				
Per	Per Functional Unit of Annualized Lane-Mile (LMY)		44	202	197
	tonne-CO <sub>2</sub> -eq. / lane-mile-year				
Per	<b>Per Functional Unit of Vehicles Mile Travelled (VMT)</b> tonne-CO <sub>2</sub> -eq. / million VMT		11.1	10.4	6.4